

1. Ground Floor Beam (D/1-2) – Secondary beam

Slab C-D/ 1-2 = 4356/1575

=2.766 (one way system)

Slab D-E/ 1-2 = 4415/4356

=1.01 (two way system)

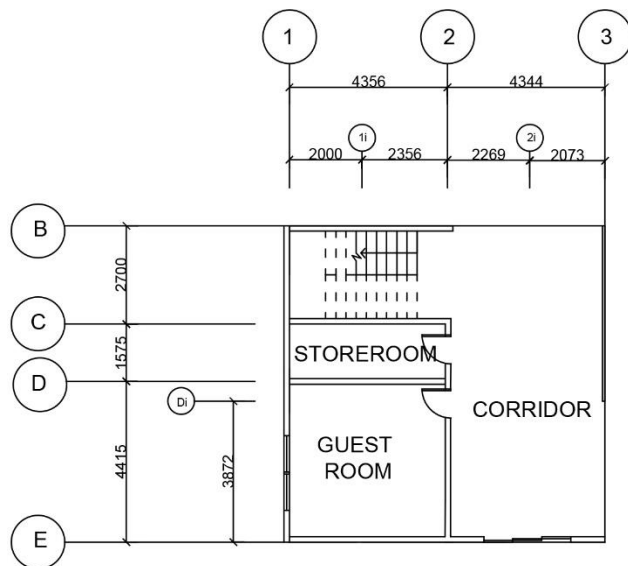
Slab thickness = 0.15m

Wall thickness = 0.15m

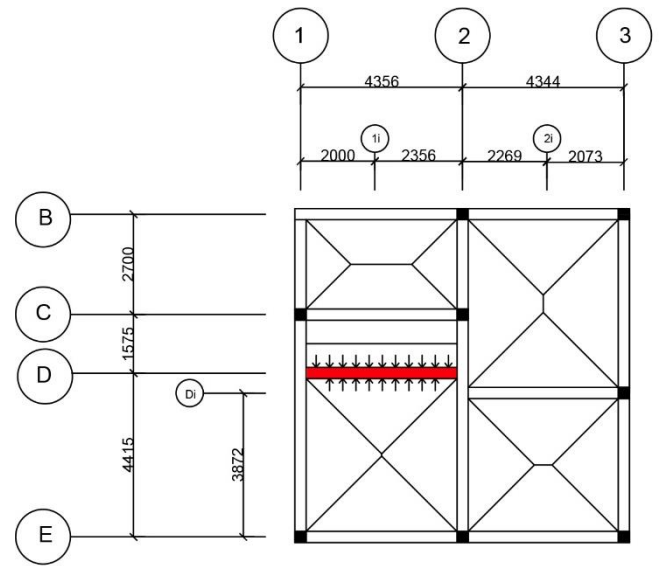
Wall height = 3.2m

Beam size = 0.15x0.3m (secondary beam)

= 0.3x0.3m (primary beam)



GROUND FLOOR PLAN (ARCHITECTURAL)

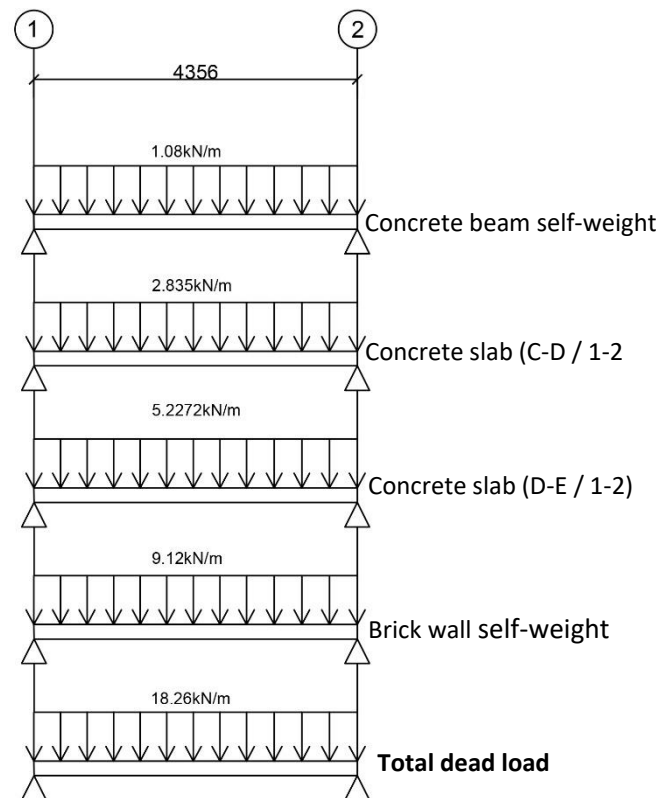


GROUND FLOOR PLAN (STRUCTURAL)

Dead Load

- Concrete beam self-weight
 $= (0.15 \times 0.3) \text{m}^2 \times 24 \text{kN/m}^3$
 $= 1.08 \text{ kN/m}$
- Concrete slab (C-D / 1-2) self-weight
 $= 0.15 \text{m} \times 24 \text{kN/m}^3 \times (1.575/2) \text{m}$
 $= 2.835 \text{ kN/m}$
- Concrete slab (D-E / 1-2) self-weight
 $= 0.15 \text{m} \times 24 \text{kN/m}^3 \times (4.356/2) \text{m} \times (2/3)$
 $= 5.2272 \text{ kN/m}$
- Brick wall self-weight
 $= (0.15 \times 3.2) \text{m}^2 \times 19 \text{kN/m}^3$
 $= 9.12 \text{ kN/m}$

Total dead load = $(1.08 + 2.835 + 5.2272 + 9.12) \text{ kN/m}$
 $= 18.26 \text{ kN/m}$



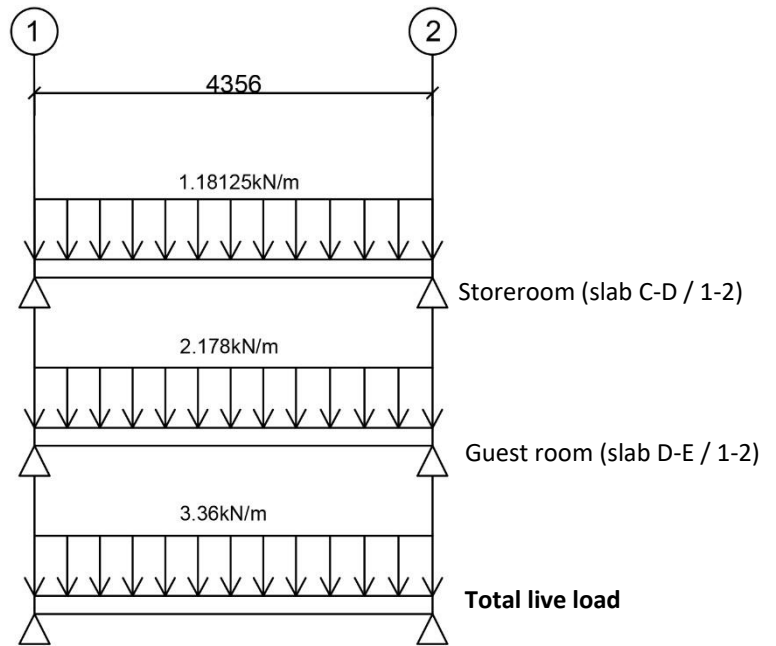
Live Load

Live load UBBL: Storeroom 1.5kN/m^2

Guest room 1.5kN/m^2

1. Storeroom (slab C-D / 1-2)
 $= 1.5\text{kN/m}^2 \times (1.575/2)\text{m}$
 $= 1.18125\text{ kN/m}$
2. Guest room (slab D-E / 1-2)
 $= 1.5\text{kN/m}^2 \times (4.356/2)\text{m} \times (2/3)$
 $= 2.178\text{ kN/m}$

Total live load $= (1.18125 + 2.178)\text{ kN/m}$
 $= 3.36\text{kN/m}$



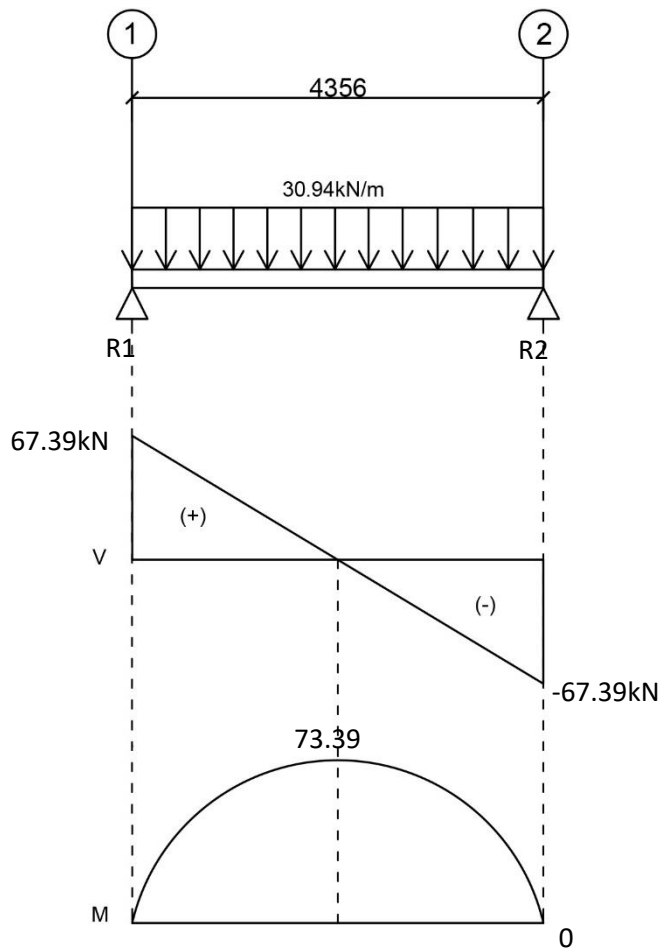
Ultimate load

Ultimate dead load $= 18.26\text{kN/m} \times 1.4$
 $= 25.564\text{kN/m}$

Ultimate live load $= 3.36\text{kN/m} \times 1.6$
 $= 5.376\text{kN/m}$

Total ultimate load $= 25.564\text{kN/m} + 5.376\text{kN/m}$
 $= 30.94\text{kN/m}$

Ultimate load diagram



$$\Sigma F_y = 0$$

$$R_1 = R_2$$

$$= (30.94 \text{ kN/m} \times 4.356 \text{ m}) / 2$$

$$= 67.39 \text{ kN (point load)}$$

$$\text{Positive area} = \text{Negative area}$$

$$= \frac{1}{2} \times 2.178 \times 67.39$$

$$= 73.39$$

2. Ground Floor Beam (C-E/2) – Primary beam

Slab B-Di/ 2-3 = 4818/4344

=1.109 (two way system)

Slab Di-E/ 2-3 = 4344/3872

=1.12 (two way system)

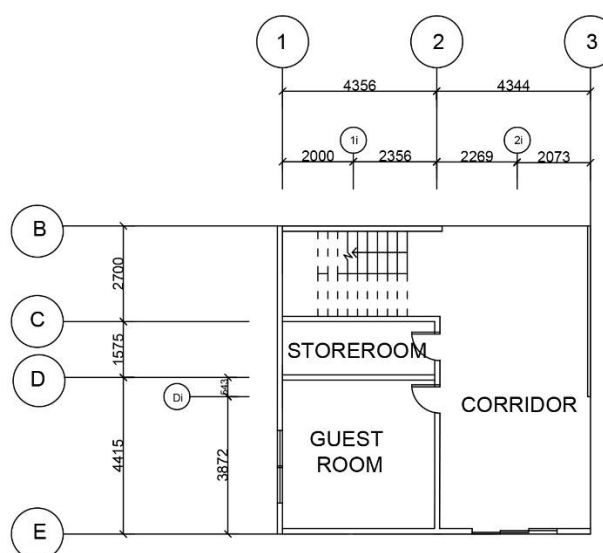
Slab thickness = 0.15m

Wall thickness = 0.15m

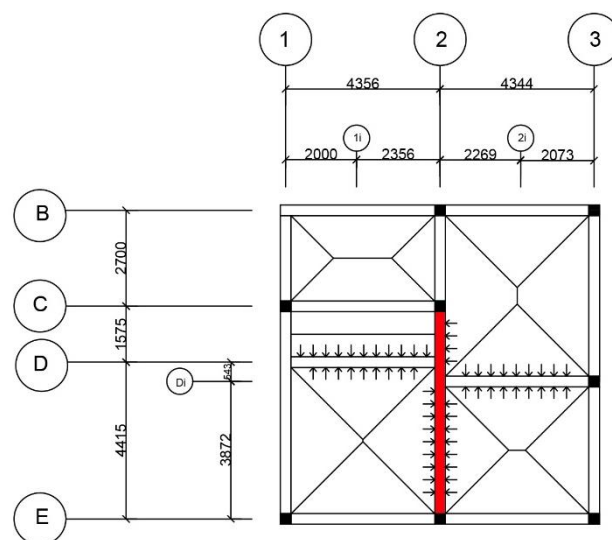
Wall height = 3.2m

Beam size = 0.15x0.3m (secondary beam)

= 0.3x0.3m (primary beam)



GROUND FLOOR PLAN (ARCHITECTURAL)



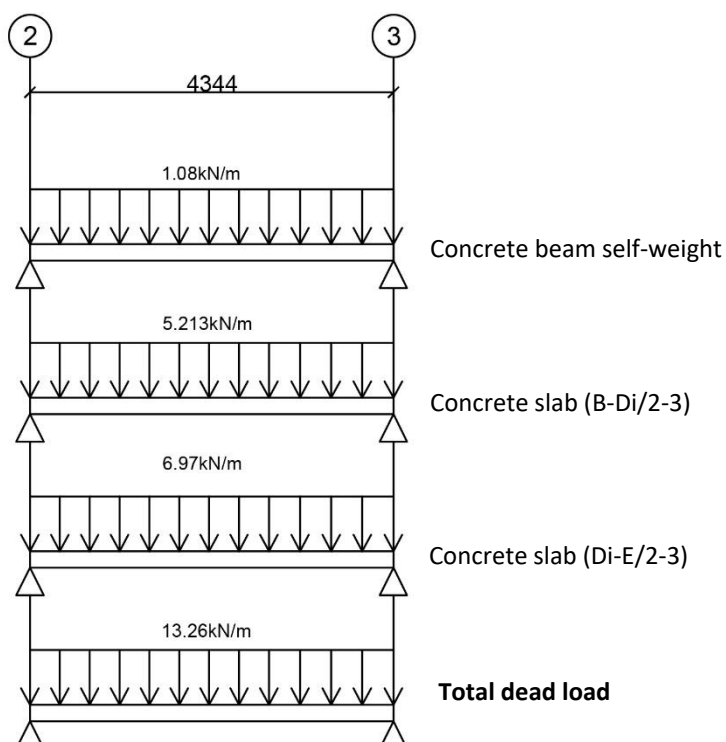
GROUND FLOOR PLAN (STRUCTURAL)

Beam (Di/2-3) – Secondary beam

Dead Load

1. Concrete beam self-weight
 $= (0.15 \times 0.3) \text{m}^2 \times 24 \text{kN/m}^3$
 $= 1.08 \text{kN/m}$
2. Concrete slab (B-Di / 2-3)
 $= 0.15 \text{m} \times 24 \text{kN/m}^3 \times (4.344/2) \text{m} \times (2/3)$
 $= 5.213 \text{kN/m}$
3. Concrete slab (Di-E / 2-3)
 $= 0.15 \text{m} \times 24 \text{kN/m}^3 \times (3.872/2) \text{m}$
 $= 6.97 \text{kN/m}$

Total dead load = $(1.08 + 5.213 + 6.97) \text{ kN/m}$
 $= 13.26 \text{kN/m}$

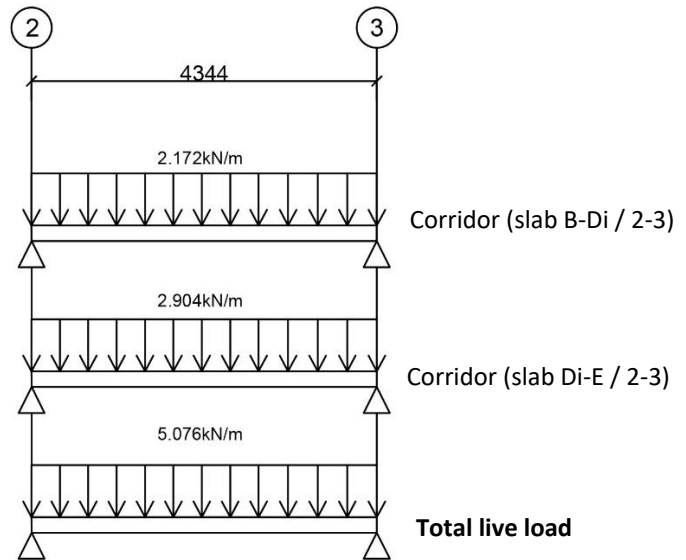


Live Load

Live load UBBL: Corridor 1.5kN/m²

1. Corridor (slab B-Di / 2-3)
= 1.5kN/m² x (4.344/2)m x (2/3)
= 2.172kN/m
2. Corridor (slab Di-E / 2-3)
= 1.5kN/m² x (3.872/2)m
= 2.904kN/m

Total live load = (2.172 + 2.904) kN/m
= 5.076kN/m



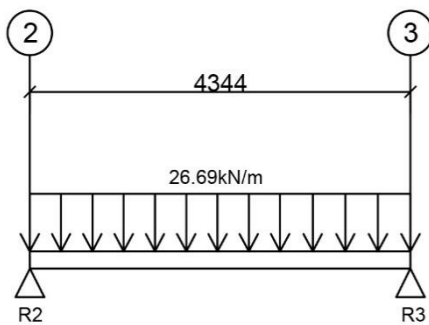
Ultimate load

Ultimate dead load = 13.26kN/m x 1.4
= 18.564kN/m

Ultimate live load = 5.076kN/m x 1.6
= 8.122kN/m

Total ultimate load = 18.564kN/m + 8.122kN/m
= 26.69kN/m

Ultimate load diagram



$$\Sigma F_y = 0$$

$$R_2 = R_3$$

$$= (26.69\text{kN/m} \times 4.344\text{m}) / 2$$

$$= 57.97\text{kN (point load)}$$

Beam (C-E/2) – Primary beam

Dead Load

1. Concrete beam self-weight
 $= (0.3 \times 0.3) \text{m}^2 \times 24 \text{kN/m}^3$
 $= 2.16 \text{kN/m}$
2. Concrete slab (B-Di / 2-3)
 $= 0.15 \text{m} \times 24 \text{kN/m}^3 \times (4.344/2) \text{m}$
 $= 7.819 \text{kN/m}$
3. Concrete slab (D-E / 1-2)
 $= 0.15 \text{m} \times 24 \text{kN/m}^3 \times (4.356/2) \text{m}$
 $= 7.84 \text{kN/m}$
4. Concrete slab (Di-E / 2-3)
 $= 0.15 \text{m} \times 24 \text{kN/m}^3 \times (3.872/2) \text{m} \times (2/3)$
 $= 4.646 \text{kN/m}$
5. Brick wall self-weight
 $= (0.15 \text{m} \times 3.2) \text{m}^2 \times 19 \text{kN/m}^3$
 $= 9.12 \text{kN/m}$

Total dead load

For C-D= $(2.16+7.819+9.12) \text{kN/m}$

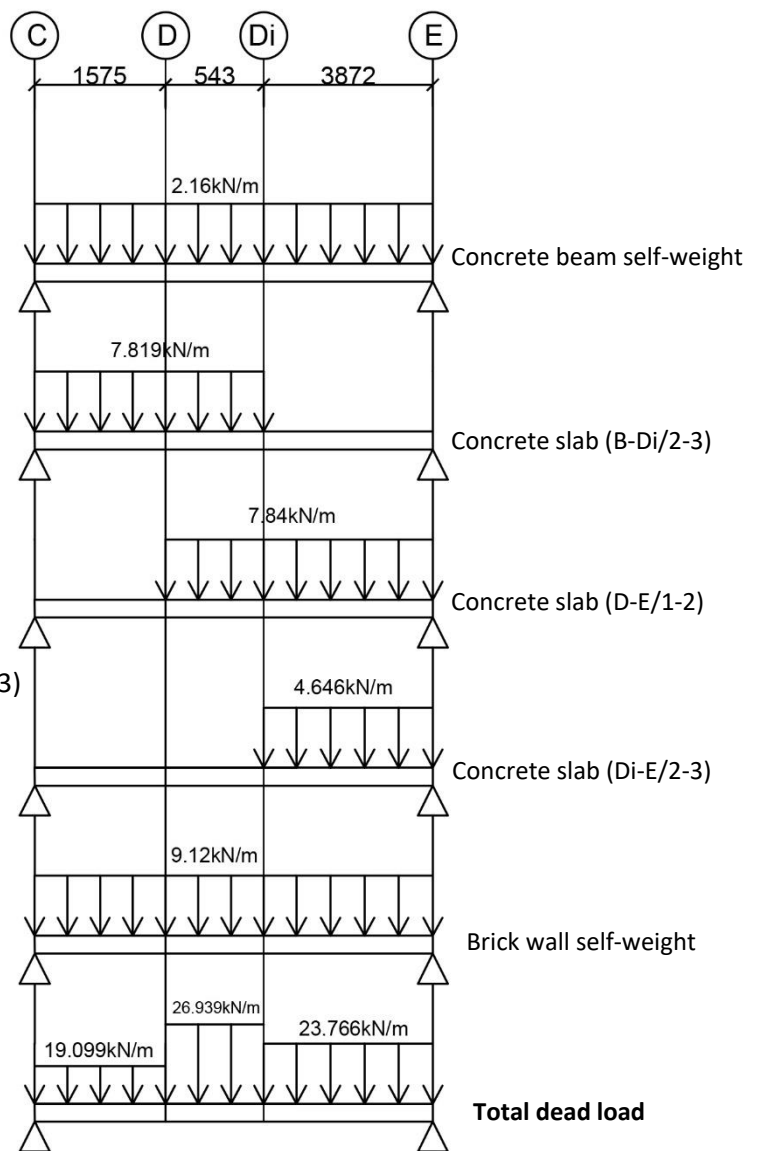
$$= 19.099 \text{kN/m}$$

For D-Di= $(2.16+7.819+7.84+9.12) \text{kN/m}$

$$= 26.939 \text{kN/m}$$

For Di-E= $(2.16+7.84+4.646+9.12) \text{kN/m}$

$$= 23.766 \text{kN/m}$$



Live Load

Live load UBBL: Corridor 1.5kN/m^2

Guest Room 1.5kN/m^2

1. Corridor (slab B-Di / 2-3)
 $= 1.5\text{kN/m}^2 \times (4.344/2)\text{m}$
 $= 3.258\text{kN/m}$
2. Guest Room (slab D-E / 1-2)
 $= 1.5\text{kN/m}^2 \times (4.356/2)\text{m}$
 $= 3.267\text{kN/m}$
3. Corridor (slab Di-E / 2-3)
 $= 1.5\text{kN/m}^2 \times (3.872/2)\text{m} \times (2/3)$
 $= 1.936\text{kN/m}$

Total live load

For C-D = 3.258 kN/m

For D-Di = $(3.258 + 3.267)\text{kN/m}$
 $= 6.525\text{kN/m}$

For Di-E = $(3.267 + 1.936)\text{kN/m}$
 $= 5.203\text{kN/m}$

Ultimate load

Ultimate dead load C-D = $19.099\text{kN/m} \times 1.4 = 26.739\text{kN/m}$

Ultimate dead load D-Di = $26.939\text{kN/m} \times 1.4 = 37.71\text{kN/m}$

Ultimate dead load Di-E = $23.766\text{kN/m} \times 1.4 = 33.27\text{kN/m}$

Ultimate live load C-D = $3.258\text{kN/m} \times 1.6 = 5.213\text{kN/m}$

Ultimate live load D-Di = $6.525\text{kN/m} \times 1.6 = 10.44\text{kN/m}$

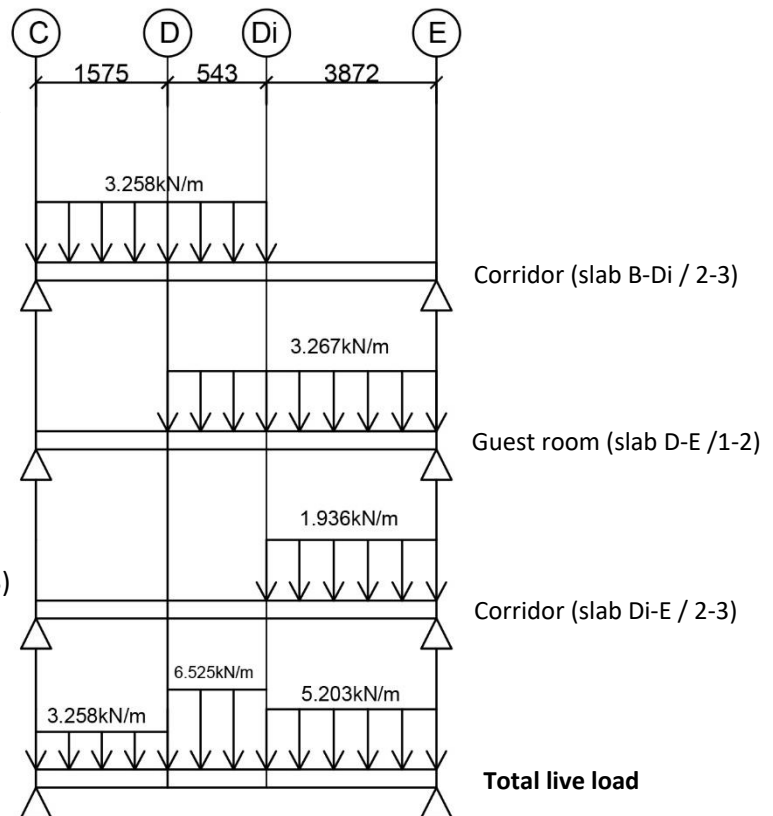
Ultimate live load Di-E = $5.203\text{kN/m} \times 1.6 = 8.325\text{kN/m}$

Total ultimate load

For C-D = $26.739 + 5.213 = 31.95\text{kN/m}$

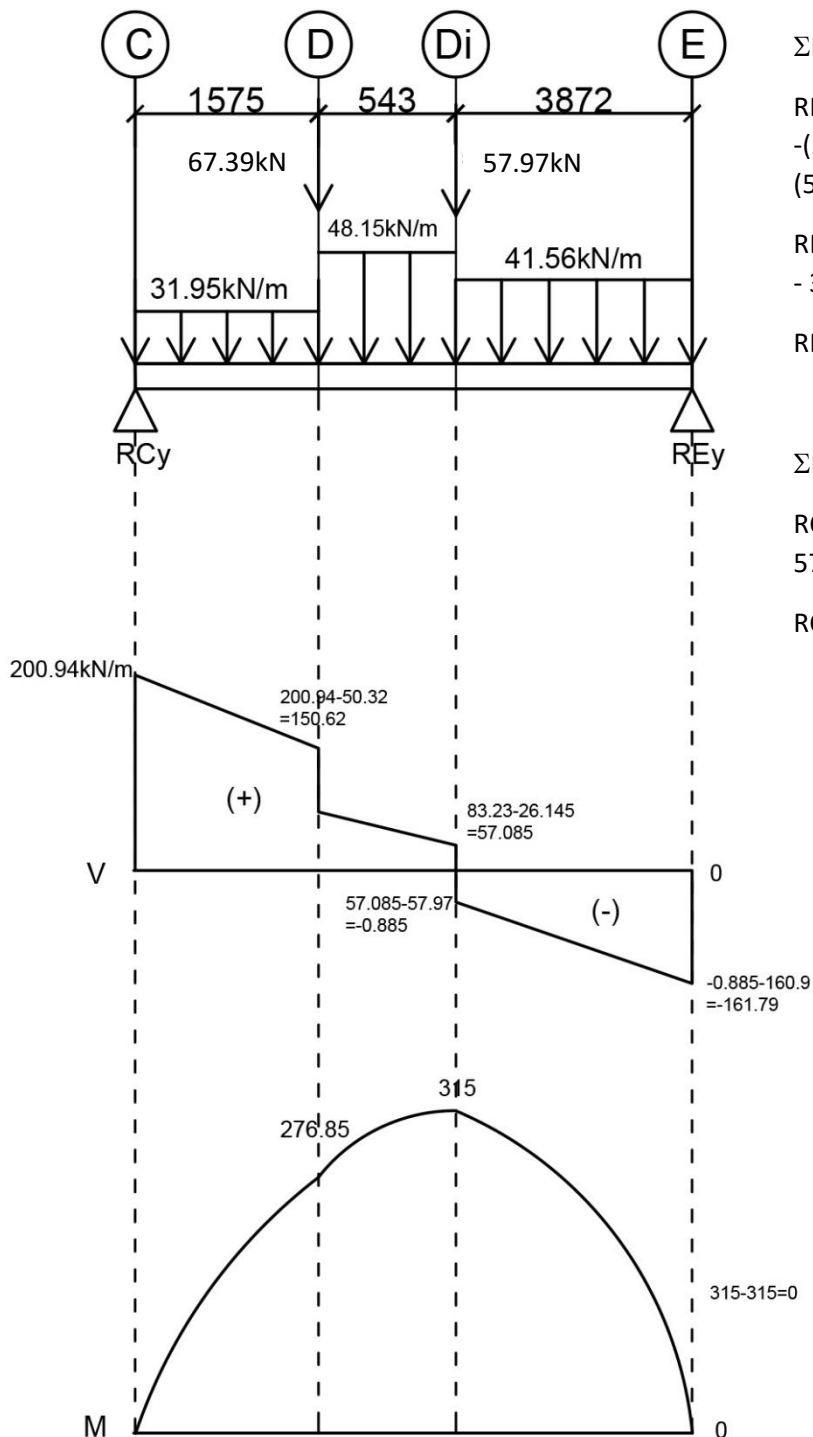
For D-Di = $37.71 + 10.44 = 48.15\text{kN/m}$

For Di-E = $33.27 + 8.325 = 41.56\text{kN/m}$



Ultimate load diagram –Beam (D/1-2) already calculated, point load=67.39kN (refer to pg.3)

–Beam (Di/2-3) already calculated, point load=57.97kN (refer to pg.5)



$$\Sigma M_c = 0$$

$$R_{Ey} (5.99) - (160.9)(4.054) - (57.97)(2.118) - (26.145)(1.847) - (67.39)(1.575) - (50.32)(0.788) = 0$$

$$R_{Ey} (5.99) - 652.29 - 122.78 - 48.29 - 106.14 - 39.65 = 0$$

$$R_{Ey} = 161.79 \text{ kN}$$

$$\Sigma F_y = 0$$

$$R_{Cy} + 161.79 - 50.32 - 67.39 - 26.145 - 57.97 - 160.9 = 0$$

$$R_{Cy} = 200.94$$

$$\begin{aligned} \text{Positive area} &= \frac{1}{2}(200.94 + 150.62)(1.575) + \\ &\quad \frac{1}{2}(83.23 + 57.085)(0.543) \\ &= 276.85 + 38.096 \\ &= 314.95 \\ &\approx 315 \end{aligned}$$

$$\begin{aligned} \text{Negative area} &= \frac{1}{2}(0.885 + 161.79)(3.872) \\ &= 314.78 \\ &\approx 315 \end{aligned}$$

3. Ground Floor Beam (C-E/1) – Primary beam

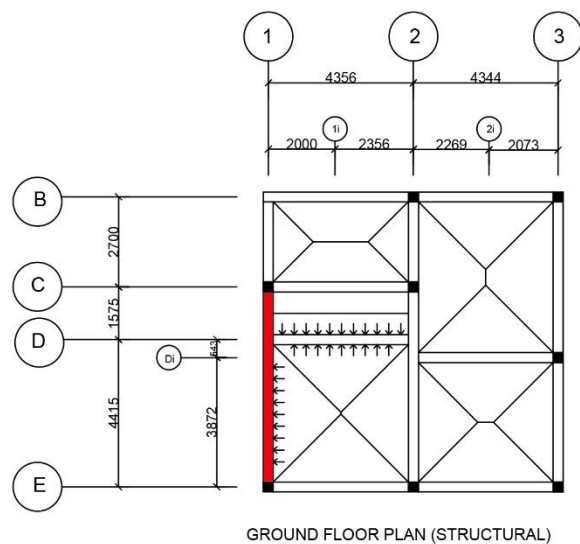
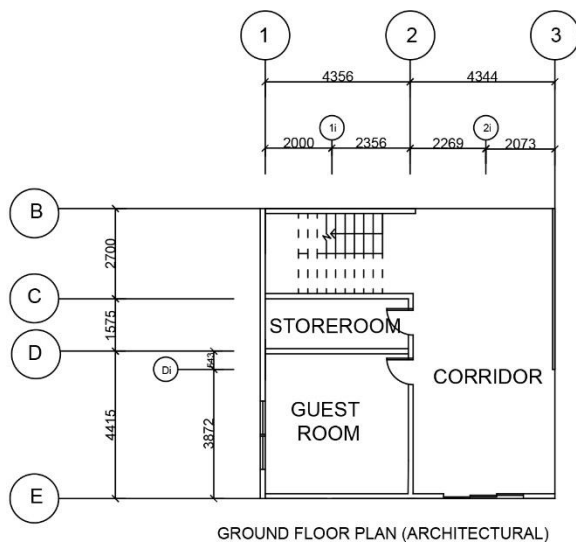
Slab thickness = 0.15m

Wall thickness = 0.15m

Wall height = 3.2m

Beam size = 0.15x0.3m (secondary beam)

= 0.3x0.3m (primary beam)



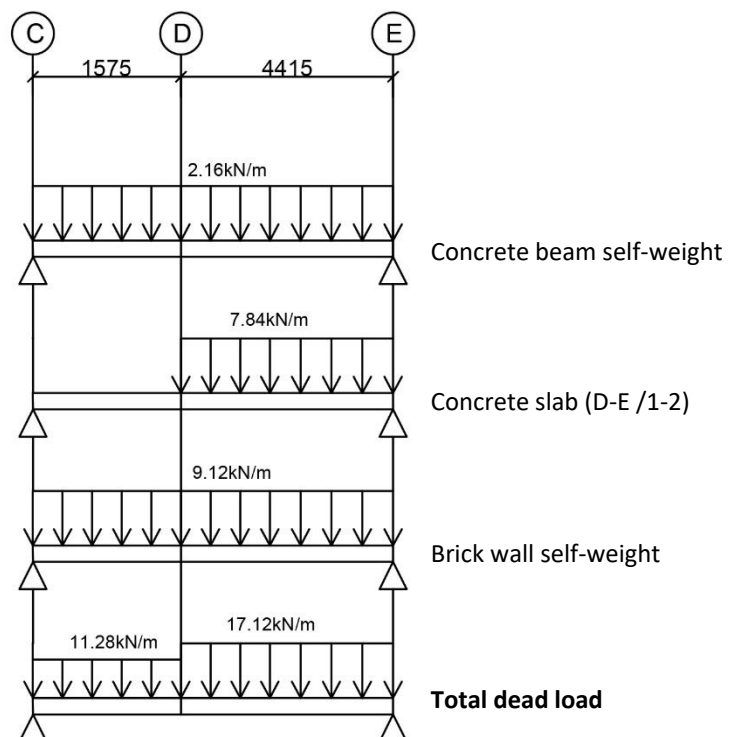
Dead Load

1. Concrete beam self-weight
 $= (0.3 \times 0.3) \text{m}^2 \times 24 \text{kN/m}^3$
 $= 2.16 \text{kN/m}$
2. Concrete slab (D-E / 1-2) self-weight
 $= 0.15 \text{m} \times (4.356/2) \text{m} \times 24 \text{kN/m}^3$
 $= 7.84 \text{kN/m}$
3. Brick wall self-weight
 $= (0.15 \times 3.2) \text{m}^2 \times 19 \text{kN/m}^3$
 $= 9.12 \text{kN/m}$

Total dead load

For C-D = $(2.16 + 9.12) \text{kN/m}$
 $= 11.28 \text{kN/m}$

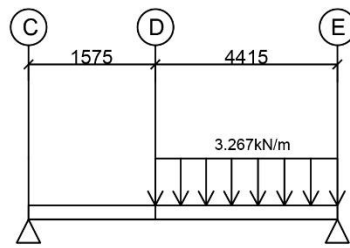
For D-E = $(2.16 + 7.84 + 9.12) \text{kN/m}$
 $= 17.12 \text{kN/m}$



Live Load

1. Guest room (slab D-E/1-2)
 $= 1.5\text{kN/m}^2 \times (4.356/2)\text{m}$
 $= 3.267\text{kN/m}$

Total live load for D-E = 3.267kN/m



Guest room (slab D-E/1-2)

Ultimate load

$$\begin{aligned}\text{Ultimate dead load C-D} &= 11.28\text{kN/m} \times 1.4 \\ &= 15.792\text{kN/m}\end{aligned}$$

$$\begin{aligned}\text{Ultimate dead load D-E} &= 17.12\text{kN/m} \times 1.4 \\ &= 23.968\text{kN/m}\end{aligned}$$

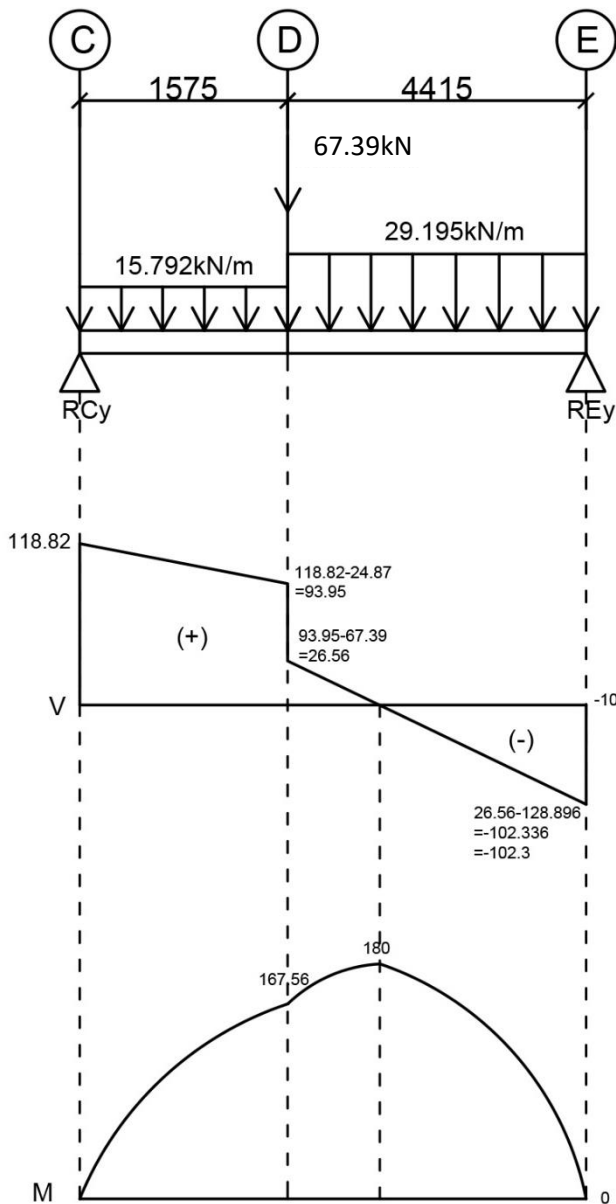
$$\begin{aligned}\text{Ultimate live load D-E} &= 3.267\text{kN/m} \times 1.6 \\ &= 5.227\text{kN/m}\end{aligned}$$

Total ultimate load

$$\text{For C-D} = 15.792\text{kN/m}$$

$$\begin{aligned}\text{For D-E} &= 23.968 + 5.227 \\ &= 29.195\text{kN/m}\end{aligned}$$

Ultimate load diagram –Beam (D/1-2) already calculated, point load=67.39kN (refer to pg.3)



$$\Sigma M_c = 0$$

$$R_{Ey} (5.99) - (128.896)(3.78) - (67.39)(1.575) - (24.87)(0.788) = 0$$

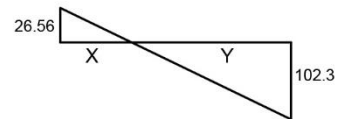
$$R_{Ey} (5.99) - 487.23 - 106.14 - 19.6 = 0$$

$$R_{Ey} = 102.332 \text{ kN}$$

$$\Sigma F_y = 0$$

$$R_{Cy} + 102.332 - 24.87 - 67.39 - 128.896 = 0$$

$$R_{Cy} = 118.82$$



$$x/y = 26.56/102.3 \text{ --- (1)}$$

$$x + y = 4.415 \text{ --- (2)}$$

$$x = 4.415 - y \text{ --- (3)}$$

sub (3) into (1),

$$(4.415 - y)102.3 = 26.56y$$

$$Y = 3.5$$

$$X = 0.915$$

$$\text{Positive area} = \frac{1}{2}(118.82 + 93.95)(1.575) +$$

$$\frac{1}{2}(0.915 \times 26.56)$$

$$= 179.71$$

$$\approx 180$$

$$\text{Negative area} = \frac{1}{2}(3.5 \times 102.3)$$

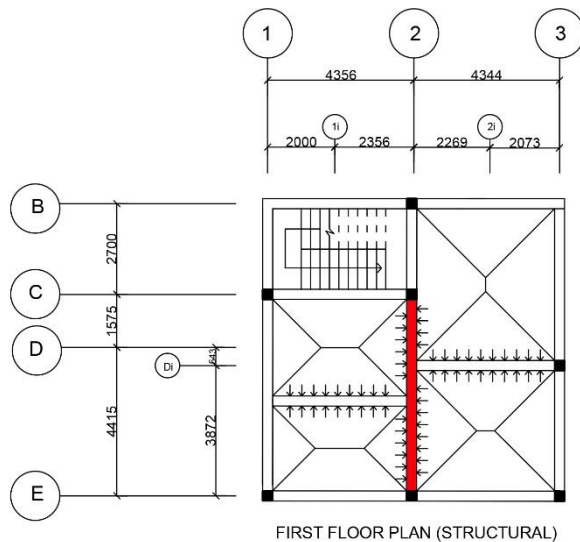
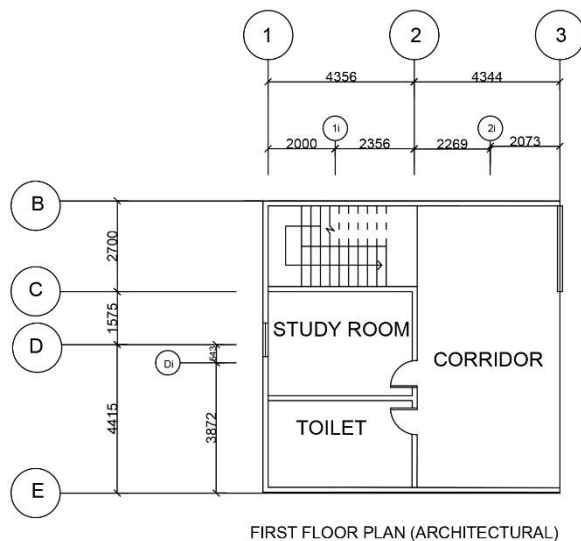
$$= 179$$

$$\approx 180$$

4. First Floor Beam (C-E/2) – Primary beam

Slab (C-Dii/1-2) = $4356/3167 = 1.37$ (Two way system)
 Slab (Dii-E/1-2) = $4356/2821 = 1.5$ (Two way system)
 Slab (B-Di/2-3) = $4818/4344 = 1.1$ (Two way system)
 Slab (Di-E/2-3) = $4344/3872 = 1.1$ (Two way system)

Slab thickness = 0.15m
 Wall thickness = 0.15m
 Wall height = 3.2m
 Beam size = 0.15x0.3m (secondary beam)
 = 0.3x0.3m (primary beam)

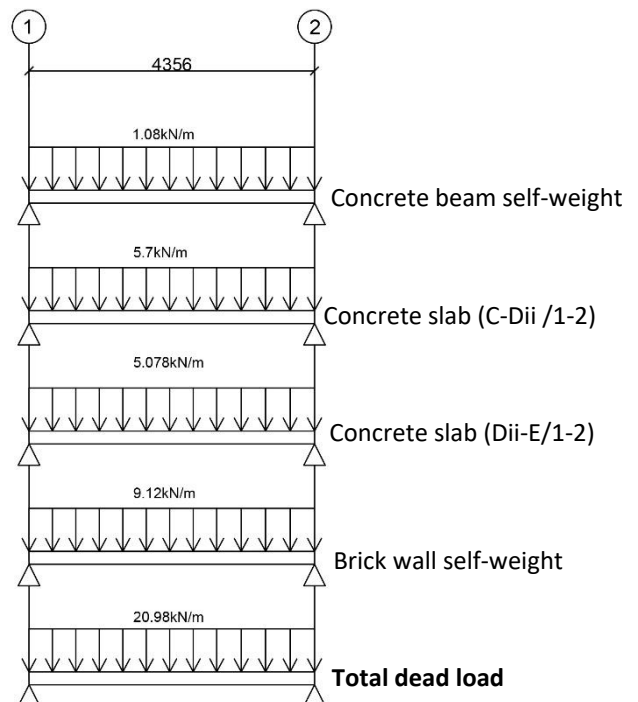


Beam (Dii/1-2) – Secondary beam

Dead Load

- Concrete beam self-weight
 $= (0.15 \times 0.3) \text{m}^2 \times 24 \text{kN/m}^3$
 $= 1.08 \text{ kN/m}$
- Concrete slab (C-Dii/1-2) self-weight
 $= 0.15 \text{m} \times 24 \text{kN/m}^3 \times (3.167/2) \text{m}$
 $= 5.7 \text{ kN/m}$
- Concrete slab (Dii-E/1-2) self-weight
 $= 0.15 \text{m} \times 24 \text{kN/m}^3 \times (2.821/2) \text{m}$
 $= 5.078 \text{ kN/m}$
- Brick wall self-weight
 $= (0.15 \times 3.2) \text{m}^2 \times 19 \text{kN/m}^3$
 $= 9.12 \text{ kN/m}$

Total dead load = $(1.08 + 5.7 + 5.078 + 9.12) \text{ kN/m}$
 $= 20.98 \text{ kN/m}$



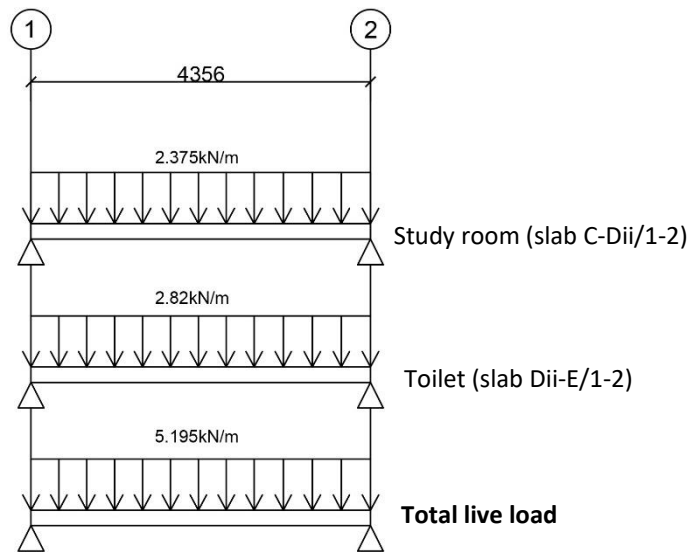
Live Load

Live load UBL: study room 1.5kN/m^2

toilet 2.0kN/m^2

1. Study room (slab C-Dii/1-2)
 $= 1.5\text{kN/m}^2 \times (3.167/2)\text{m}$
 $= 2.375\text{kN/m}$
2. Toilet (slab Dii-E/1-2)
 $= 2.0\text{kN/m}^2 \times (2.821/2)\text{m}$
 $= 2.82\text{kN/m}$

Total live load $= (2.375 + 2.82)\text{kN/m}$
 $= 5.195\text{kN/m}$



Ultimate load

Ultimate dead load 1-2 $= 20.98\text{kN/m} \times 1.4$

$$= 29.372\text{kN/m}$$

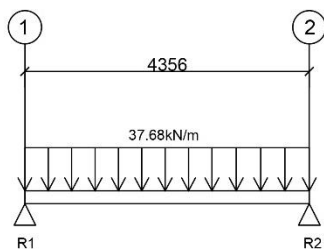
Ultimate live load 1-2 $= 5.195\text{kN/m} \times 1.6$

$$= 8.312\text{kN/m}$$

Total ultimate load 1-2 $= 29.372 + 8.312$

$$= 37.68\text{kN/m}$$

Ultimate load diagram



$$\Sigma F_y = 0$$

$$R_1 = R_2$$

$$= (37.68\text{kN/m} \times 4.356\text{m}) / 2$$

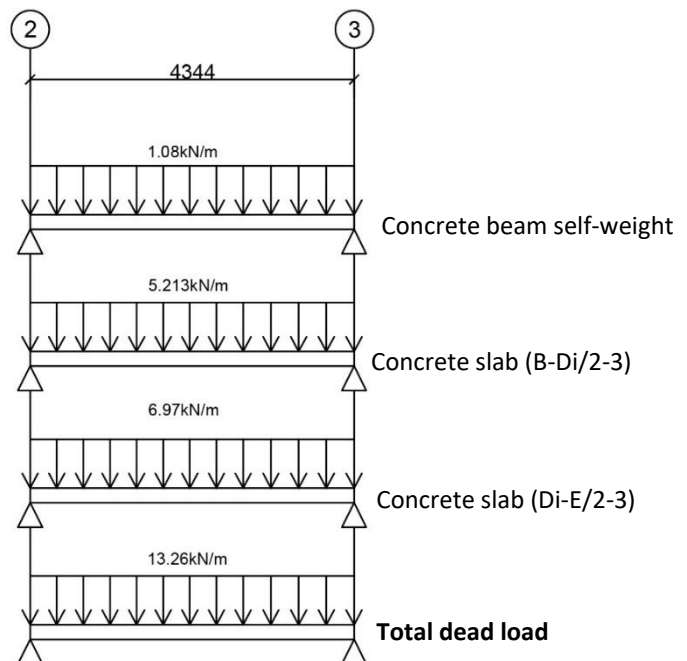
$$= 82.07\text{kN (point load)}$$

Beam (Di/2-3) – Secondary beam

Dead Load

1. Concrete beam self-weight
 $= (0.15 \times 0.3) \text{m}^2 \times 24 \text{kN/m}^3$
 $= 1.08 \text{ kN/m}$
2. Concrete slab (B-Di/2-3) self-weight
 $= 0.15 \text{m} \times 24 \text{kN/m}^3 \times (4.344/2) \times (2/3)$
 $= 5.213 \text{ kN/m}$
3. Concrete slab (Di-E/2-3) self-weight
 $= 0.15 \text{m} \times 24 \text{kN/m}^3 \times (3.872/2)$
 $= 6.97 \text{ kN/m}$

Total dead load $= 1.08 + 5.213 + 6.97$
 $= 13.26 \text{ kN/m}$

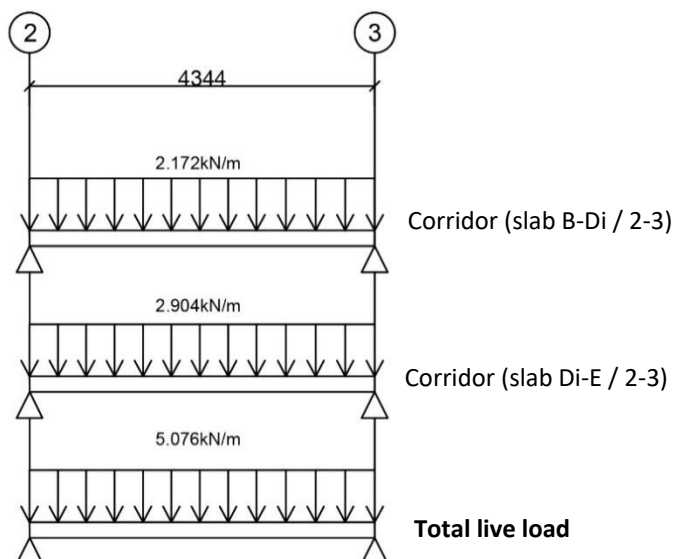


Live Load

Live load UBBL: Corridor 1.5 kN/m²

1. Corridor (slab B-Di / 2-3)
 $= 1.5 \text{ kN/m}^2 \times (4.344/2) \text{m} \times (2/3)$
 $= 2.172 \text{ kN/m}$
2. Corridor (slab Di-E / 2-3)
 $= 1.5 \text{ kN/m}^2 \times (3.872/2) \text{m}$
 $= 2.904 \text{ kN/m}$

Total live load $= (2.172 + 2.904) \text{ kN/m}$
 $= 5.076 \text{ kN/m}$



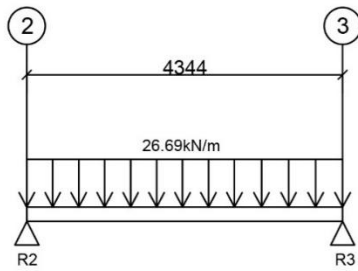
Ultimate load

Ultimate dead load $= 13.26 \text{ kN/m} \times 1.4$
 $= 18.564 \text{ kN/m}$

Ultimate live load $= 5.076 \text{ kN/m} \times 1.6$
 $= 8.122 \text{ kN/m}$

Total ultimate load $= 18.564 \text{ kN/m} + 8.122 \text{ kN/m}$
 $= 26.69 \text{ kN/m}$

Ultimate load diagram



$$\Sigma F_y = 0$$

$$R_2 = R_3$$

$$= (26.69 \text{ kN/m} \times 4.344 \text{ m}) / 2$$

$$= 57.97 \text{ kN (point load)}$$

Beam (C-E/2) - Primary beam

Dead Load

1. Concrete beam self-weight
 $= (0.3 \times 0.3) \text{ m}^2 \times 24 \text{ kN/m}^3$
 $= 2.16 \text{ kN/m}$
2. Concrete slab (C-Dii/1-2)
 $= 0.15 \text{ m} \times 24 \text{ kN/m}^3 \times (3.167/2) \times (2/3)$
 $= 3.8 \text{ kN/m}$
3. Concrete slab (Dii-E/1-2)
 $= 0.15 \text{ m} \times 24 \text{ kN/m}^3 \times (2.2821/2) \times (2/3)$
 $= 3.385 \text{ kN/m}$
4. Concrete slab (B-Di/2-3)
 $= 0.15 \text{ m} \times 24 \text{ kN/m}^3 \times (4.344/2)$
 $= 7.819 \text{ kN/m}$
5. Concrete slab (Di-E/2-3)
 $= 0.15 \text{ m} \times 24 \text{ kN/m}^3 \times (3.872/2) \times (2/3)$
 $= 4.646 \text{ kN/m}$
6. Brick wall self-weight
 $= (0.15 \times 3.2) \text{ m}^2 \times 19 \text{ kN/m}^3$
 $= 9.12 \text{ kN/m}$

Total dead load

$$\text{For C-Di} = 2.16 + 3.8 + 7.819 + 9.12$$

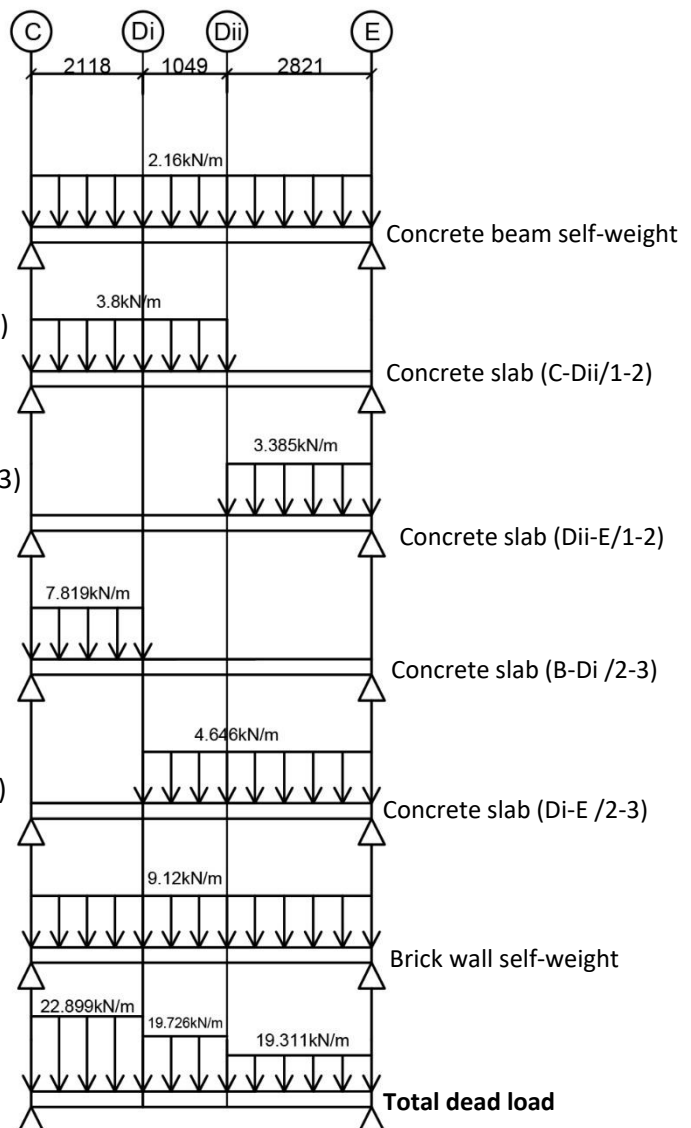
$$= 22.899 \text{ kN/m}$$

$$\text{For Di-Dii} = 2.16 + 3.8 + 4.646 + 9.12$$

$$= 19.726 \text{ kN/m}$$

$$\text{For Dii-E} = 2.16 + 3.385 + 4.646 + 9.12$$

$$= 19.311 \text{ kN/m}$$



Live Load

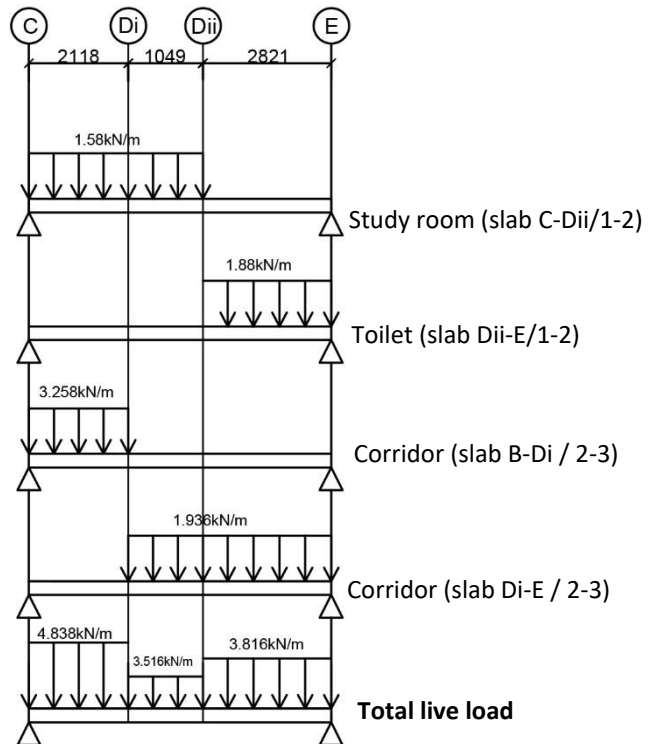
1. Study room (slab C-Dii/1-2)
 $= 1.5\text{kN/m}^2 \times (3.167/2)\text{m} \times (2/3)$
 $= 1.58\text{kN/m}$
2. Toilet (slab Dii-E/1-2)
 $= 2.0\text{kN/m}^2 \times (2.821/2)\text{m} \times (2/3)$
 $= 1.88\text{kN/m}$
3. Corridor (slab B-Di/2-3)
 $= 1.5\text{kN/m}^2 \times (4.344/2)\text{m}$
 $= 3.258\text{kN/m}$
4. Corridor (slab Di-E/2-3)
 $= 1.5\text{kN/m}^2 \times (3.872/2)\text{m} \times (2/3)$
 $= 1.936\text{kN/m}$

Total live load

FOR C-Di $= 1.58 + 3.258 = 4.838\text{kN/m}$

FOR D-Dii $= 1.58 + 1.936 = 3.516\text{kN/m}$

FOR Dii-E $= 1.88 + 1.936 = 3.816\text{kN/m}$



Ultimate load

Ultimate dead load for C-Di $= 22.899\text{kN/m} \times 1.4 = 32.059\text{kN/m}$

Ultimate dead load for Di-Dii $= 19.726\text{kN/m} \times 1.4 = 27.616\text{kN/m}$

Ultimate dead load for Dii-E $= 19.311\text{kN/m} \times 1.4 = 27.035\text{kN/m}$

Ultimate live load for C-Di $= 4.838\text{kN/m} \times 1.6 = 7.74\text{kN/m}$

Ultimate live load for Di-Dii $= 3.516\text{kN/m} \times 1.6 = 5.626\text{kN/m}$

Ultimate live load for Dii-E $= 3.816\text{kN/m} \times 1.6 = 6.106\text{kN/m}$

Total Ultimate load

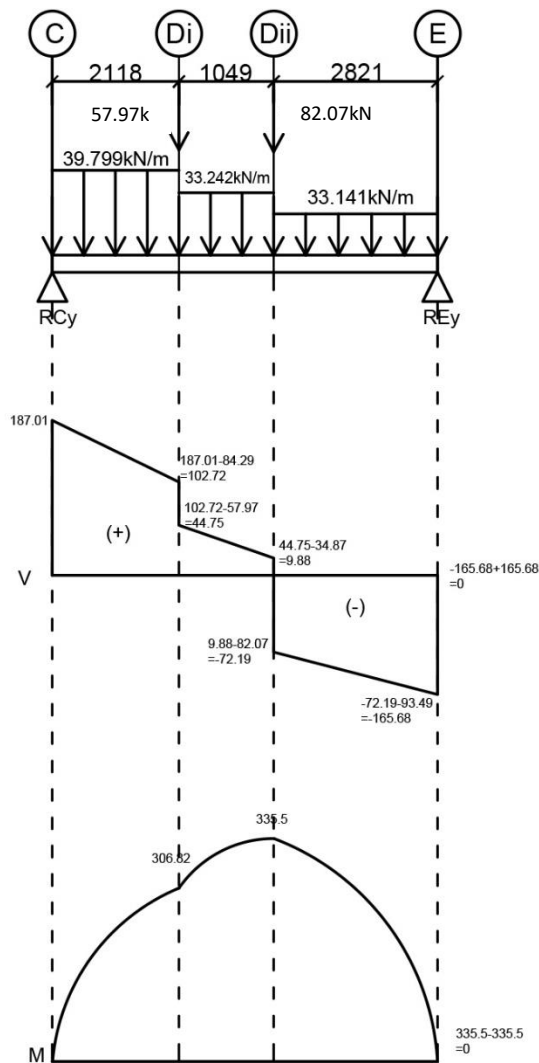
For C-Di $= 32.059 + 7.74 = 39.799\text{kN/m}$

For Di-Dii $= 27.616 + 5.626 = 33.242\text{kN/m}$

For Dii-E $= 27.035 + 6.106 = 33.141\text{ kN/m}$

Ultimate load diagram –Beam (Dii/1-2) already calculated, point load=82.07kN (refer to pg.13)

–Beam (Di/2-3) already calculated, point load=57.97kN (refer to pg.15)



$$\Sigma Mc=0$$

$$REy (5.99) - (93.49)(4.578) - (82.07)(3.167) - (34.87)(2.643) - (57.97)(2.118) - (84.29)(1.059)=0$$

$$REy (5.99) - 427.997 - 259.92 - 92.16 - 122.78 - 89.26=0$$

$$REy = 165.68kN$$

$$\Sigma Fy=0$$

$$RCy + 165.68 - 93.49 - 82.07 - 34.87 - 57.97 - 84.29 = 0$$

$$RCy = 187.01$$

$$\text{Positive area} = \frac{1}{2}(187.01+102.72)(2.118)+$$

$$\frac{1}{2}(44.75+9.88)(1.049)$$

$$=335.49$$

$$\approx 335.5$$

$$\text{Negative area} = \frac{1}{2}(72.19+165.68)(2.821)$$

$$= 335.5$$

5. First Floor Beam (C-E/1) – Primary beam

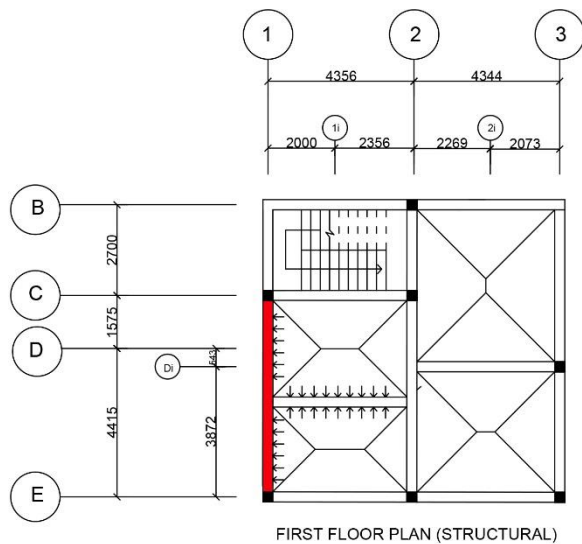
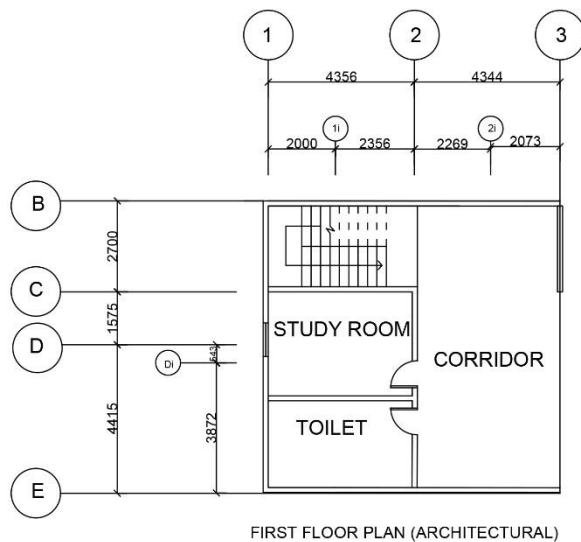
Slab thickness = 0.15m

Wall thickness = 0.15m

Wall height = 3.2m

Beam size = 0.15x0.3m (secondary beam)

= 0.3x0.3m (primary beam)



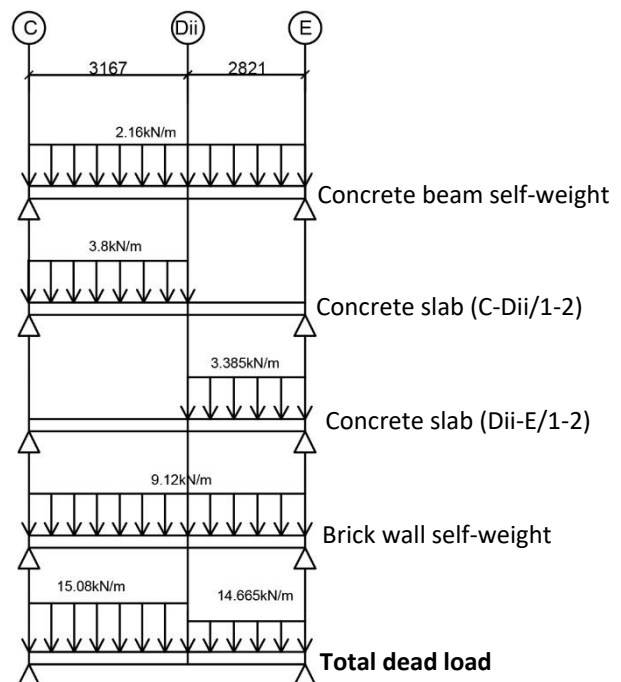
Dead Load

1. Concrete beam self-weight
 $= (0.3 \times 0.3) \text{m}^2 \times 24 \text{kN/m}^3$
 $= 2.16 \text{kN/m}$
2. Concrete slab (C-Dii/1-2) self-weight
 $= 0.15 \text{m} \times (3.167/2) \text{m} \times 24 \text{kN/m}^3 \times (2/3)$
 $= 3.8 \text{kN/m}$
3. Concrete slab (Dii-E/1-2) self-weight
 $= 0.15 \text{m} \times (2.821/2) \text{m} \times 24 \text{kN/m}^3$
 $= 3.385 \text{kN/m}$
4. Brick wall self-weight
 $= (0.15 \times 3.2) \text{m}^2 \times 19 \text{kN/m}^3$
 $= 9.12 \text{kN/m}$

Total dead load

For C-Dii = $(2.16 + 3.8 + 9.12) \text{kN/m} = 15.08 \text{kN/m}$

For Dii-E = $(2.16 + 3.385 + 9.12) \text{kN/m} = 14.665 \text{kN/m}$



Live Load

Live load UBBL: Study room 1.5kN/m^2

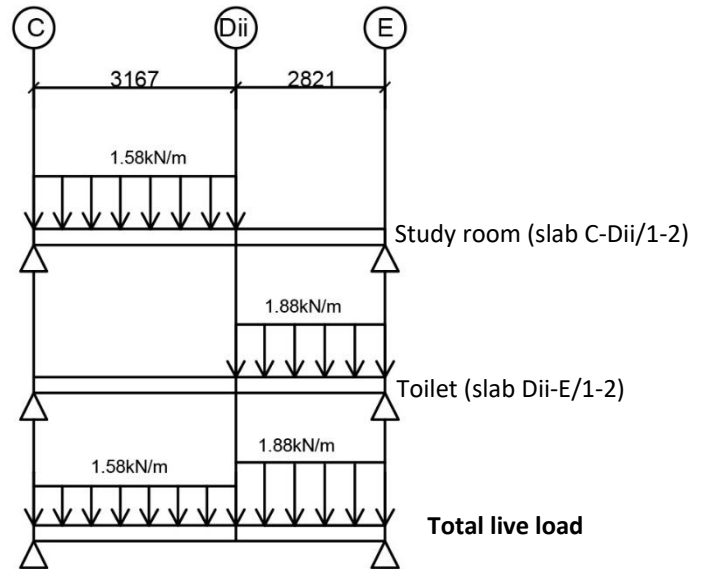
Toilet 2.0kN/m^2

1. Study room (slab C-Dii/1-2)
 $= 1.5\text{kN/m}^2 \times (3.167/2)\text{m} \times (2/3)$
 $= 1.58\text{kN/m}$
2. Toilet (slab Dii-E/1-2)
 $= 2.0\text{kN/m}^2 \times (2.821/2)\text{m} \times (2/3)$
 $= 1.88\text{kN/m}$

Total live load for

For C-Dii = 1.58kN/m

For Dii-E = 1.88kN/m



Ultimate load

Ultimate dead load C-Dii = $15.08\text{kN/m} \times 1.4$

$= 21.112\text{kN/m}$

Ultimate dead load Dii-E = $14.665\text{kN/m} \times 1.4$

$= 20.531\text{kN/m}$

Ultimate live load C-Dii = $1.58\text{kN/m} \times 1.6$

$= 2.528\text{kN/m}$

Ultimate live load Dii-E = $1.88\text{kN/m} \times 1.6$

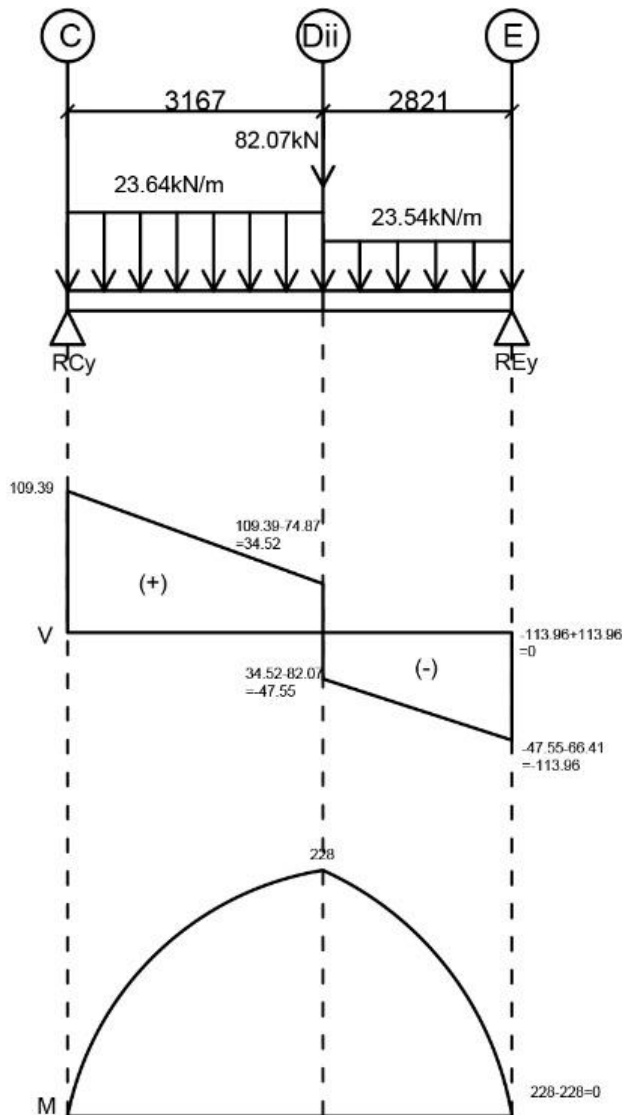
$= 3.008\text{kN/m}$

Total ultimate load

For C-Dii = $(21.112 + 2.528)\text{kN/m} = 23.64\text{kN/m}$

For Dii-E = $(20.531 + 3.008)\text{kN/m} = 23.54\text{kN/m}$

Ultimate load diagram –Beam (Dii/1-2) already calculated, point load=82.07kN (refer to pg.13)



$$\Sigma M_c = 0$$

$$R_{Ey} (5.99) - (66.41)(4.58) - (82.07)(3.167) - (74.87)(1.58) = 0$$

$$R_{Ey} (5.99) - 304.158 - 259.92 - 118.29 = 0$$

$$R_{Ey} = 113.96 \text{ kN}$$

$$\Sigma F_y = 0$$

$$R_{Cy} + 113.96 - 74.87 - 82.07 - 66.41 = 0$$

$$R_{Cy} = 109.39$$

$$\text{Positive area} = \frac{1}{2}(109.39 + 34.52)(3.167)$$

$$= 227.88$$

$$\approx 228$$

$$\text{Negative area} = \frac{1}{2}(47.55 + 113.96)(2.821)$$

$$= 227.81$$

$$\approx 228$$

6. First Floor Beam (B-Di/3)- Secondary beam

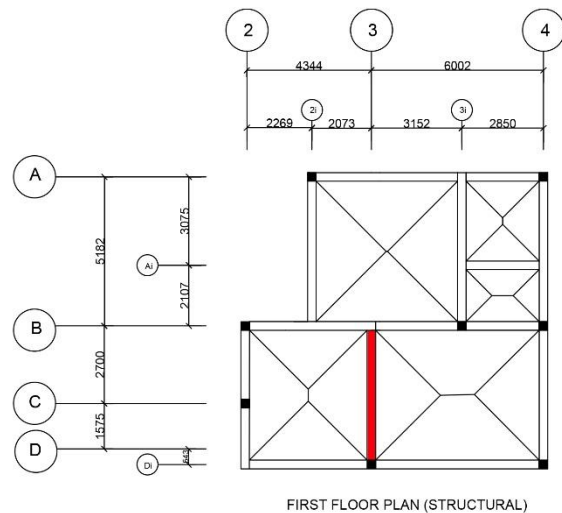
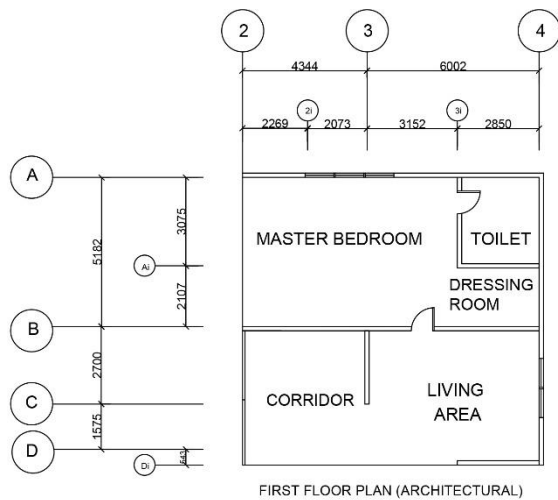
Slab thickness = 0.15m

Wall thickness = 0.15m

Wall height = 3.2m

Beam size = 0.15x0.3m (secondary beam)

= 0.3x0.3m (primary beam)



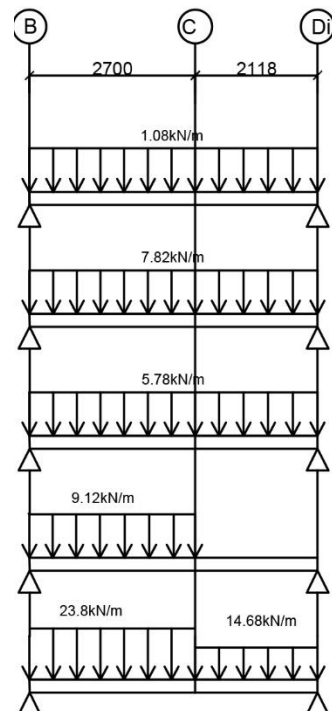
Dead Load

1. Concrete beam self-weight
 $= (0.3 \times 0.15) \text{m}^2 \times 24 \text{kN/m}^3$
 $= 1.08 \text{ kN/m}$
2. Concrete slab (B-Di / 2-3)
 $= 0.15 \text{m} \times 24 \text{kN/m}^3 \times (4.344/2) \text{m}$
 $= 7.82 \text{ kN/m}$
3. Concrete slab (B-Di / 3-4)
 $= 0.15 \text{m} \times 24 \text{kN/m}^3 \times (4.818/2) \text{m} \times (2/3)$
 $= 5.78 \text{ kN/m}$
4. Brick wall self-weight
 $= (0.15 \times 3.2) \text{m}^2 \times 19 \text{kN/m}^3$
 $= 9.12 \text{ kN/m}$

Total dead load

For B-C = $1.08 + 7.82 + 5.78 + 9.12 = 23.8 \text{ kN/m}$

For C-Di = $1.08 + 7.82 + 5.78 = 14.68 \text{ kN/m}$



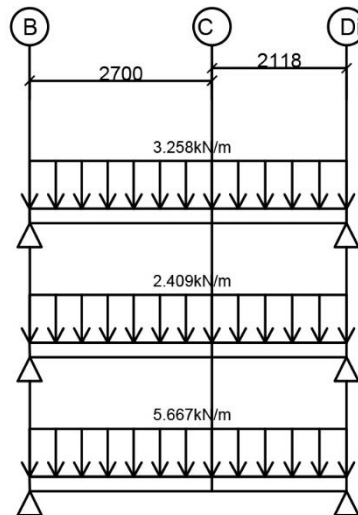
Live Load

Live load UBBL: Corridor 1.5kN/m^2

Living area 1.5kN/m^2

1. Corridor (slab B-Di/2-3)
 $= 1.5\text{kN/m}^2 \times (4.344/2)\text{m}$
 $= 3.258\text{kN/m}$
2. Living area (slab B-Di/3-4)
 $= 1.5\text{kN/m}^2 \times (4.818/2)\text{m} \times (2/3)$
 $= 2.409\text{kN/m}$

Total live load $= 3.258 + 2.409$
 $= 5.667\text{ kN/m}$



Ultimate load

Ultimate dead load B-C $= 23.8\text{kN/m} \times 1.4$
 $= 33.32\text{kN/m}$

Ultimate dead load C-Di $= 14.68\text{kN/m} \times 1.4$
 $= 20.552\text{kN/m}$

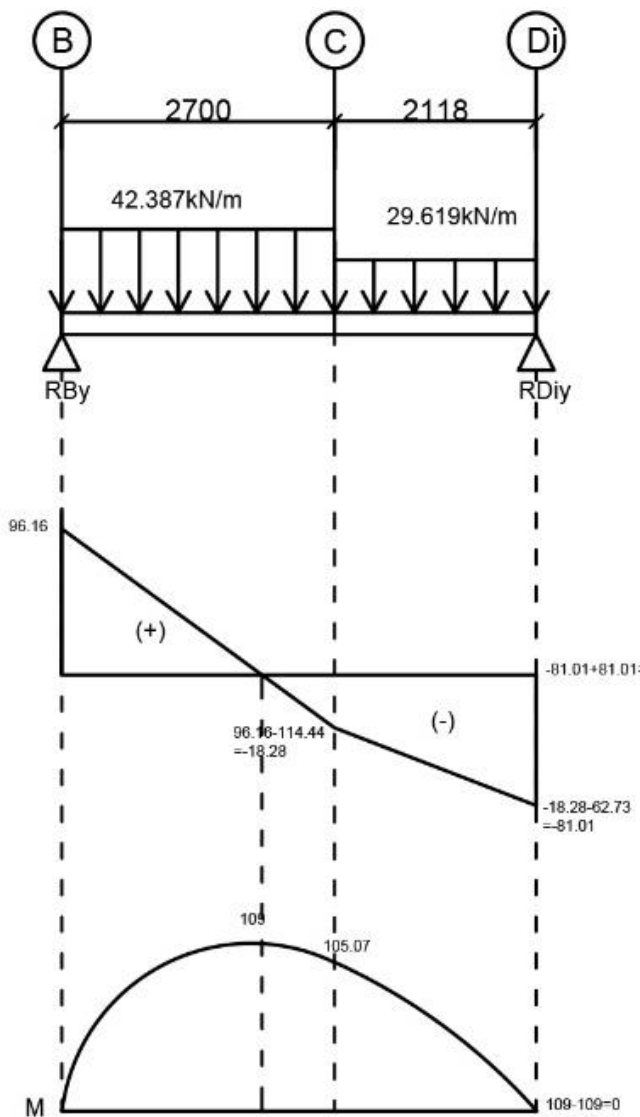
Ultimate live load B-Di $= 5.667\text{kN/m} \times 1.6$
 $= 9.067\text{kN/m}$

Total ultimate load

For B-C $= (33.32 + 9.067)\text{kN/m} = 42.387\text{kN/m}$

For C-Di $= (20.552 + 9.067)\text{kN/m} = 29.619\text{kN/m}$

Ultimate load diagram



$$\Sigma M_B = 0$$

$$R_{Di} (4.818) - (62.73)(3.759) - (114.44)(1.35) = 0$$

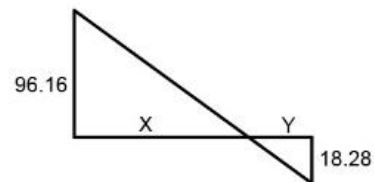
$$R_{Di} (4.818) - 235.8 - 154.49 = 0$$

$$R_{Di} = 81.01 \text{ kN}$$

$$\Sigma F_y = 0$$

$$R_{By} + 81.01 - 114.44 - 62.73 = 0$$

$$R_{By} = 96.16$$



$$x/y = 96.16/18.28 \text{ --- (1)}$$

$$x + y = 2.7 \text{ --- (2)}$$

$$x = 2.7 - y \text{ --- (3)}$$

sub (3) into (1),

$$(2.7 - y)(18.28) = 96.16y$$

$$Y = 0.43$$

$$X = 2.27$$

$$\text{Positive area} = \frac{1}{2}(2.27)(96.16)$$

$$= 109.14$$

$$\approx 109$$

$$\text{Negative area} = \frac{1}{2}(0.43)(18.28) +$$

$$\frac{1}{2}(18.28 + 81.01)(2.118)$$

$$= 3.93 + 105.15$$

$$= 109$$